## Program: B.E. Civil Engineering

Curriculum Scheme: Revised 2016

Examination: Fourth Year Semester: VIII

## Course Code CE C801 and Course Name: Design and Drawing of Reinforced Concrete Structures

|           | MODULE 1 COMPREHENSIVE DESIGN OF BUILDING                                      |
|-----------|--|
|           |  |
| 1.1       | The section of singly reinforced beam in which the permissible stress in steel |
|           | and concrete reaches earlier than that in concrete is called                   |
| Option A: | Under reinforced section   |
| Option B: | Over reinforced section  |
| Option C: | balanced section   |
| Option D: | Economic section   |
|           |  |
| 1.2       | The Partial factor of safety for steel in LSM may be taken as                  |
| Option A: | 1.5  |
| Option B: | 1.15   |
| Option C: | 1.78   |
| Option D: | 3  |
|           |  |
| 1.3       | Characteristic strength is defined as the value of strength below which not    |
|           | more that% of the test results are expected to lie.                            |
| Option A: | 5  |
| Option B: | 15   |
| Option C: | 25   |
| Option D: | 50   |
|           |  |
| 1.4       | The design Strength of Concrete is taken as in Limit State of                  |
|           | Collapse   |
| Option A: | 0.45fck  |
| Option B: | 0.67fck  |

| Option C: | Fck   |
|-----------|---|
| Option D: | 0.23fck   |
|           |   |
| 1.5       | Partial safety factor in case of dead load for stability against overturning or |
|           | stress reversal is  |
| Option A: | 1.2   |
| Option B: | 0.9   |
| Option C: | 0.7   |
| Option D: | 2.3   |
|           |   |
| 1.6       | Live load comprises of  |
| Option A: | Permanently attached loads  |
| Option B: | Temporarily attached loads whose value and position may change                  |
| Option C: | Permanent as well as temporary loads  |
| Option D: | Snow loads  |
|           |   |
| 1.7       | The balance moment of resistance of the singly reinforced beam effective        |
|           | depth of beam is 450 mm having is 139.73 kNm. If M20 concrete and Fe 415        |
|           | steel are used ,the width of the section is                                     |
| Option A: | 250mm   |
|           |   |
| Option B: | 200 mm  |
| Option C: | 300 mm  |
| Option D: | 350 mm  |
| 1.0       |   |
| 1.8       | A beam of cross section of 200mm *450mm and is subjected to bending             |
|           | moment of 135 kNm. If M20 concrete and Fe250 steel are used, beam should        |
|           | be designed as  |
| Option A: | Singly reinforced beam  |
| Option B: | Doubly reinforced beam  |
| Option C: | Singly as well as doubly reinforced beam  |
| Option D: | Singly reinforced beam with more steel  |
|           |   |

| 1.9       | An isolated T beam has an effective span of 4800 mm and flange width of    |
|-----------|--|
|           | 800 mm. the flange thickness is 130 mm and the rib is 300 mm wide. The     |
|           | effective flange width is  |
| Option A: | 1000mm   |
| option in |  |
| Option B: | 780 mm   |
| Option C: | 350 mm   |
| Option D: | 450 mm   |
| 1.10      | For a T beam, if main reinforcement of slab must be                        |
|           |  |
| Option A: | parallel to beam,  |
| Option B: | Perpendicular to beam  |
| Option C: | Inclined to axis of beam at 30 degrees                                     |
| Option D: | Partly parallel partly perpendicular                                       |
|           |  |
| 1.11      | A simply supported beam has 350mm width and 500 mm effective depth.        |
|           | The beam subjected to a factored shear force of 62.5 kN. The nominal shear |
|           | stress in Mpa is   |
| Option A: | 0.15   |
| Option B: | 0.35   |
| Option C: | 0.50   |
| Option D: | 0.75   |
|           |  |
| 1.12      | A beam 300 mm* 600 mm is subjected to factored bending moment of 115       |
|           | kNm and factored torsion 45 kNm. The equivalent bending moment is          |
| Option A: | 194.41 kNm.  |
| Option B: | 102.54 kNm   |
| Option C: | 322.12 kNm   |

| Option D: | 112.95kNm  |
|-----------|--|
|           |  |
| 1.13      | A beam 300 mm* 600 mm is subjected to factored shear force 95 kN and           |
|           | factored torsion 45 kNm. The equivalent ultimate shear is                      |
| Option A: | 100 kN   |
| Option B: | 235 kN   |
| Option C: | 335 kN   |
| Option D: | 475 kN   |
|           |  |
| 1.14      | What is the max spacing of stirrups for a beam of effective depth 400 mm.      |
|           | mm for   |
| Option A: | 100 mm   |
| Option B: | 150 mm   |
| Option C: | 300 mm   |
| Option D: | 450 mm   |
|           |  |
| 1.15      | The load on footing is 1650kN inclusive of its own weight. If safe bearing     |
|           | capacity of soil is 100 kN per sq. meter. The diameter of circular footing are |
| Option A: | 4.58 m   |
| Option B: | 5.12 m   |
| Option C: | 8.19 m   |
| Option D: | 1.1 m  |
|           |  |
| 1.16      | What is shear resisted by a bent up bar of 16 mm diameter of Fe415 steel.      |
| Option A: | 72.21 kN   |
| Option B: | 51.06 kN   |
| Option C: | 87.81 kN   |
| Option D: | 100.23 kN  |
|           |  |
| 1.17      | Depths of different beams are given. Which of these beams needs side face      |
|           | reinforcement.   |
| Option A: | 350 mm   |

| Option B: | 450 mm   |
|-----------|--|
| Option C: | 950 mm   |
| Option D: | 600 mm   |
|           |  |
| 1.18      | For a one way slab the area of main reinforcement required is 300 mm. find   |
|           | spacing (centre to centre distance) for 8 mm bar.                            |
| Option A: | 250 mm   |
| Option B: | 125 mm   |
| Option C: | 166 mm   |
| Option D: | 400 mm   |
|           |  |
| 1.19      | For deflection control of slab, the basic span to effective depth ratio for  |
|           | cantilever slab is   |
| Option A: | 7  |
| Option B: | 20   |
| Option C: | 26   |
| Option D: | 40   |
|           |  |
| 1.20      | In case of one way slab, the main reinforcement is                           |
| Option A: | Along shorter span   |
| Option B: | Along longer span  |
| Option C: | Along both shorter and longer spans  |
| Option D: | At corners only  |
|           |  |
| 1.21      | The depth of slab is 250 mm. the Fe 415 distribution steel is provided. Area |
|           | of distribution steel in sq mm is  |
| Option A: | 300  |
| Option B: | 400  |
| Option C: | 150  |
| Option D: | 100  |

| 1.22      | If for Columns with helical reinforcement, if the requirement for ratio of the |
|-----------|--|
|           | volume of helical reinforcement to the volume of the core is satisfied then    |
|           | Load Carrying capacity of column is increased by percent compared to           |
|           | similar column with lateral tie.   |
| Option A: | 5  |
| Option B: | 4  |
| Option C: | 6  |
| Option D: | 7  |
|           |  |
| 1.23      | A RCC short column is 400mm*400 mm is carrying a factored load of 1800         |
|           | kN. If M20 concrete and Fe 415 steel are used, the area of steel required in   |
|           | sq. mm is  |
| Option A: | 1287   |
| Option B: | 869  |
| Option C: | 1926   |
| Option D: | 2541   |
|           |  |
| 1.24      | A RCC short column is 400mm*425 mm is carrying a load of 1195kN. If            |
|           | M20 concrete and Fe 415 steel are used, the area of steel required in sq. mm   |
|           | is   |
| Option A: | 1287   |
| Option B: | 869  |
| Option C: | 1560   |
| Option D: | 2541   |
|           |  |
| 1.25      | The load on footing is 1650kN inclusive of its own weight. If safe bearing     |
|           | capacity of soil is 150 kN per sq. meter. The dimensions of square footing are |
| Option A: | 3.32m*3.32m  |
| Option B: | 2.42m *2.52 m  |
| Option C: | 1.43m*1.43m  |

| Option D: | 2.81m*2.81m  |
|-----------|--|
|           | MODULE 2 STAIRCASE   |
| 2.1       | The pitch of stair should never exceed   |
| Option A: | 20°  |
| Option B: | 25°  |
| Option C: | 30°  |
| Option D: | 40°  |
| 2.2       | A series of steps without any platform, break or landing in their direction, is called |
| Option A: | Riser  |
| Option B: | Tread  |
| Option C: | Flight   |
| Option D: | Nosing   |
|           |  |
| 2.3       | Live load on stairs not subjected to overcrowding iskN/m <sup>2</sup>                  |
| Option A: | 1.5  |
| Option B: | 6  |
| Option C: | 3  |
| Option D: | 5  |
|           |  |
| 2.4       | Landing is provided in stairs for  |
| Option A: | Increasing length of stair   |
| Option B: | To make staircase economical   |
| Option C: | For comfort of users   |
| Option D: | To reduce load   |
|           |  |
| 2.5       | For dog legged stair case floor to floor height is 3.2 m, rise: 160 mm,                |
|           | tread:250mm, depth of waist slab: 200 mm, L.L = 3 kN/Sq.m, F.F= 1                      |
|           | kN/Sq.m, total working load on stair case is about                                     |
| Option A: | 18 kN/m <sup>2</sup>   |

| Option B: | 12 kN/m²  |
|-----------|---|
| Option C: | 16 kN/m²  |
| Option D: | 20 kN/m²  |
|           |   |
| 2.6       | Choose correct value of tread and width of staircase for residential building.                  |
| Option A: | 250mm and 600 mm  |
| Option B: | 250 mm and 1200 mm  |
| Option C: | 350mm and 700 mm  |
| Option D: | 150 mm and 1000mm   |
|           |   |
| 2.7       | Live loads on stairs for dwelling houses liable to overcrawding shall be                        |
| Option A: | $2 \text{ kN/m}^2$  |
| Option B: | $2.5 \text{ kN/m}^2$  |
| Option C: | 3 kN/m²   |
| Option D: | 5 kN/m²   |
|           |   |
|           | MODULE 3 RETAINING WALL   |
|           | The safe bearing capacity of soil is 120kN/m <sup>2</sup> , unit weight of soil is              |
| 3.1       | 18kN/m <sup>3</sup> and angle of repose is 30 <sup>0</sup> degrees. Minimum depth of foundation |
|           | as per Rankine's formula is   |
| Option A: | 0.25 m  |
| Option B: | 0.50 m  |
| Option C: | 0.74 m  |
| Option D: | 1.00 m  |
|           |   |
| 3.2       | Cantilever retaining walls can safely be used for a height not more than                        |
| Option A: | 3m  |
| Option B: | 4m  |
| Option C: | 5m  |
| Option D: | 6m  |
|           |   |
| 3.3       | Which one of the following is the correct statement about retaining wall                        |

| Option A: | Toe slab and heel slab are provided at top face                               |
|-----------|---|
| Option B: | Toe slab and heel slab are provided with reinforcement at bottom face         |
| Option C: | Toe slab is provided reinforcement at top face and heel slab at bottom face   |
| Option D: | Toe slab is provided with reinforcement at bottom face and heel slab at top   |
|           | face  |
|           |   |
| 3.4       | Weep holes provided into retaining wall for the purpose of                    |
| Option A: | To provide drainage   |
| Option B: | To prevent cracks   |
| Option C: | To avoid friction behind the wall   |
| Option D: | To improve appearance   |
|           |   |
| 3.5       | The shear key is provided to  |
| Option A: | Avoid sliding failure of the wall   |
| Option B: | Improve appearance  |
| Option C: | Increase passive resistance   |
| Option D: | To resist overturning   |
|           |   |
| 3.6       | Weight of a retaining wall is 200 kN, coefficeient of friction is 0.65,       |
|           | horizontal soil pressure force per metre run of wall is 100 kN. The factor of |
|           | safety against sliding is   |
| Option A: | 1.3   |
| Option B: | 1.97  |
| Option C: | 1.74  |
| Option D: | 2.21  |
|           |   |
| 3.7       | The minimum depth of foundation depends upon                                  |
| Option A: | Safe bearing capacity of soil   |
| Option B: | Width of stem   |
| Option C: | Provision of weep holes   |
| Option D: | Reinforcement in toe slab   |
|           |   |
| 1         | •   |

| 3.8       | Weight of a retaining wall is 142 kN, coefficeient of friction is 0.6,            |
|-----------|---|
|           | horizontal soil pressure force per metre run of wall is 54 kN. The factor of      |
|           | safety against sliding is   |
| Option A: | 1.58  |
| Option B: | 2.16  |
| Option C: | 3.18  |
| Option D: | 1.21  |
|           |   |
| 3.9       | In counterfort retaining walls the upright slab                                   |
| Option A: | Acts like cantilever  |
| Option B: | Like fixed beam   |
| Option C: | As a continuous slab  |
| Option D: | Simply supported beam   |
|           |   |
| 3.10      | To have pressure wholly compressive under the base of a retaining wall of         |
|           | width b, the resultant of the weight of the wall and the pressure exerted by the  |
|           | retained, earth should have eccentricity not more than                            |
| Option A: | b/3   |
| Option B: | b/6   |
| Option C: | b/4   |
| Option D: | b/8   |
|           |   |
| 3.11      | Cantilever retaining walls can safely be used for a height not more than          |
| Option A: | 3m  |
| Option B: | 4m  |
| Option C: | 5m  |
| Option D: | бт  |
|           |   |
| 3.12      | Total pressure on the vertical face of a retaining wall of height h acts parallel |
|           | to free surface and from the base at a distance of                                |
| Option A: | h/4   |
| Option B: | 2h/3  |
| Option C: | h/3   |

| Option D: | h/2  |
|-----------|--|
|           |  |
| 3.13      | Minimum grade of concrete for retaining wall is                              |
| Option A: | M20  |
| Option B: | M25  |
| Option C: | M30  |
| Option D: | M40  |
|           |  |
| 3.14      | The heel slab of a retaining wall is subjected to factored bending moment of |
|           | 229 kNm. If effective depth of slab is 490 mm, the area of steel required is |
|           | mm2. (use M20 concrete and Fe 415 steel)                                     |
| Option A: | 1521   |
| Option B: | 1834   |
| Option C: | 1372   |
| Option D: | 2738   |
|           | Module 4 WATER LANK  |
| 4.1       | In case of the circular water tank with flexible base, due to internal water |
|           | pressure the wall is subjected to hoop force equal to                        |
|           | $(\Upsilon = \text{sp. weight of water, H= depth D= diameter of tank})$      |
| Option A: | Υ H (D /2)   |
| Option B: | ΥН   |
| Option C: | Υ H2   |
| Option D: | ΥD   |
|           |  |
| 4.2       | Haunch reinforcement is provided in circular tanks at corners to avoid       |
| Option A: | Moment   |
| Option B: | Couple   |
| Option C: | Absolute pressure  |
| Option D: | Bursting pressure  |
| 4.3       | A movement joint which allows the adjoining parts of a structure to slide    |
|           | relative to each other with minimum restraint is known as                    |
| Option A: | Sliding joint  |

| Option C: Construction joint  4.4 What will be the hoop force if unit weight of water=Y=9.81KN/m3, height of tank=H= 5m, Diameter of circular tank= D= 10m.  Option A: 125 Kn  Option B: 383 kN  Option C: 245 kN  Option D: 90 kN  4.5 A rectangular water tank is resting on ground. If pull in wall at a level is 58860 N, the area of steel required to resist pull is mm2. (Use Fe415 steel)  Option A: 392 |
|--|
| 4.4 What will be the hoop force if unit weight of water=Y=9.81KN/m3, height of tank=H= 5m, Diameter of circular tank= D= 10m.  Option A: 125 Kn  Option B: 383 kN  Option C: 245 kN  Option D: 90 kN  4.5 A rectangular water tank is resting on ground. If pull in wall at a level is 58860 N, the area of steel required to resist pull is mm2. (Use Fe415 steel)  |
| of tank=H= 5m, Diameter of circular tank= D= 10m.  Option A: 125 Kn  Option B: 383 kN  Option C: 245 kN  Option D: 90 kN  4.5 A rectangular water tank is resting on ground. If pull in wall at a level is 58860 N, the area of steel required to resist pull is mm2. (Use Fe415 steel)  |
| of tank=H= 5m, Diameter of circular tank= D= 10m.  Option A: 125 Kn  Option B: 383 kN  Option C: 245 kN  Option D: 90 kN  4.5 A rectangular water tank is resting on ground. If pull in wall at a level is 58860 N, the area of steel required to resist pull is mm2. (Use Fe415 steel)  |
| Option A: 125 Kn  Option B: 383 kN  Option C: 245 kN  Option D: 90 kN  4.5 A rectangular water tank is resting on ground. If pull in wall at a level is 58860 N, the area of steel required to resist pull is mm2 . ( Use Fe415 steel)   |
| Option B: 383 kN  Option C: 245 kN  Option D: 90 kN  4.5 A rectangular water tank is resting on ground. If pull in wall at a level is 58860 N, the area of steel required to resist pull is mm2 . ( Use Fe415 steel)   |
| Option C: 245 kN  Option D: 90 kN  4.5 A rectangular water tank is resting on ground. If pull in wall at a level is 58860 N, the area of steel required to resist pull is mm2 . ( Use Fe415 steel)   |
| Option D: 90 kN  4.5 A rectangular water tank is resting on ground. If pull in wall at a level is 58860 N, the area of steel required to resist pull is mm2 . ( Use Fe415 steel)   |
| 4.5 A rectangular water tank is resting on ground. If pull in wall at a level is 58860 N, the area of steel required to resist pull is mm2 . ( Use Fe415 steel)  |
| 58860 N, the area of steel required to resist pull is mm2 . ( Use Fe415 steel)   |
| 58860 N, the area of steel required to resist pull is mm2 . ( Use Fe415 steel)   |
| steel)   |
|  |
| Option A: 392  |
|  |
| Option B: 492  |
| Option C: 183  |
| Option D: 256  |
|  |
| 4.6 Wall of a circular water tank with flexible base is 265 mm thick. The vertical   |
| distribution steel required is mm2.  |
| Option A: 125  |
| Option B: 418  |
| Option C: 795  |
| Option D: 129  |
|  |
| 4.7 If front counterfort are not provided then toe slab is designed as   |
| Option A: Cantilever slab  |
| Option B: Simply supported slab  |
| Option C: Fixed slab   |
| Option D: Continuous slab  |
|  |

| 4.8       | A rectangular water tank is resting on ground. If pull in wall at a level is |
|-----------|--|
|           | 49050 N, the area of steel required to resist pull is mm2 . ( Use Mild       |
|           | steel steel)   |
| Option A: | 392  |
| Option B: | 427  |
| Option C: | 183  |
| Option D: | 256  |
|           |  |
| 4.9       | For a water tank of size 4m*9m, the longer wall is designed as               |
| Option A: | Vertical cantilevers   |
| Option B: | Walls fixed at both ends   |
| Option C: | Horizontal cantilevers   |
| Option D: | Walls simply supported at ends.  |
|           |  |
| 4.10      | If front counterfort are provided then toe slab is designed as               |
| Option A: | Cantilever slab  |
| Option B: | Simply supported slab  |
| Option C: | Fixed slab   |
| Option D: | Continuous slab  |
|           |  |
| 4.11      | An elevated water tank is provided so that                                   |
| Option A: | Water can be provided at gravity pressure to large population                |
| Option B: | To reduce water pressure   |
| Option C: | To reduce soil pressure on walls of tank                                     |
| Option D: | To reduce cost of tank   |
|           |  |
| 4.12      | Net load on heel slab is   |
| Option A: | Downward load  |
| Option B: | Upward load  |
| Option C: | Horizontal load  |
| Option D: | Vertically upward load   |
|           |  |
|           | 1  |

| 4.13      | The circular water tank with rigid base, the upper portion of wall near top is              |
|-----------|---|
|           | having predominantly  |
| Option A: | Simply supported action   |
| Option B: | Cantilever action   |
| Option C: | hoop action   |
| Option D: | Sliding action  |
|           |   |
| 4.14      | The circular water tank with rigid base, the lower portion of wall near base is             |
|           | having predominantly  |
| Option A: | Simply supported action   |
| Option B: | Cantilever action   |
| Option C: | Bending action  |
| Option D: | Sliding action  |
|           |   |
| 4.15      | For circular water tank capacity of tank 800m <sup>3</sup> , depth of water tank is limited |
|           | to H=5m, then what will be the diameter of circular water tank?                             |
| Option A: | 14.27m  |
| Option B: | 203.71m   |
| Option C: | 28.54m  |
| Option D: | 7.85m   |
|           |   |
| 4.16      | Heel slab of a counterfort retaining wall is designed as                                    |
| Option A: | Continuous horizontal slab  |
| Option B: | Continuous vertical slab  |
| Option C: | Simply supported slab   |
| Option D: | Fixed slab  |
|           |   |
| 4.17      | A water tank wall is subjected to a hoop tension of 132788 N. Find spacing of               |
|           | 12 mm bars to resist this tension.(MS bars)   |
| Option A: | 95  |
| Option B: | 134   |
| Option C: | 45  |

| Option D: | 252  |
|-----------|--|
|           |  |
| 4.18      | In IS code approximate method table for shear force coefficients, for design   |
|           | of water tank  |
| Option A: | Positive sign for shear shows inward shear   |
| Option B: | Positive sign shows out word shear   |
| Option C: | Positive sign shows out downwords shear  |
| Option D: | Negative sign shows inward shear   |
| 4.19      | Circular vistar for smaller conscisios are not professed as  |
| Option A: | Circular water for smaller capacities are not preferred as   |
| Option B: | They do not look good The cost of formwork offsets the saving of materials   |
| Option C: | Circular tanks are structurally inefficient  |
| Option D: | Rectangular tanks are water tight  |
| Option D. | Rectangular tanks are water tight  |
| 4.20      | For design of elevated water tank the bending moment due to horizontal   |
|           | thrust is taken as P=lateral force, y= vertical distance from hinge.   |
| Option A: | Py/4   |
| Option B: | Py/3   |
| Option C: | Py/6   |
| Option D: | Py/12  |
|           |  |
| 4.21      | To avoid cracks in concrete  |
| Option A: | A high permissible tensile stress is adopted in steel.   |
| Option B: | A low permissible tensile stress is adopted in steel   |
| Option C: | Concrete is allowed to reach its max permissible tensile stress.   |
| Option D: | Factor of safety against cracking is kept high compared to factor of safety required for structural safety.  |
|           | MODULE 5 EARTHOLIANE REGISTANT REGIS |
|           | MODULE 5 EARTHQUAKE RESISTANT DESIGN OF STRUCTURES   |
| 5.1       | Which of the following statements best describes the state of earthquake   |
|           | prediction?  |
| Option A: | scientists can accurately predict the time and location of almost all  |
|           | earthquakes  |
| Option B: | scientists can accurately predict the time and location of about 50% of all  |
|           | earthquakes  |
| Option C: | scientists can accurately predict the time and location of about 50% of all  |
| F 1-2-2-0 | earthquakes  |
| Option D: | scientists can characterize the seismic risk of an area, but can not yet   |
| option B. |  |
|           | accurately predict most earthquakes  |

| 5.2       | State which statement is correct.   |
|-----------|---|
| Option A: | Most earthquakes can be predicted   |
| Option B: | The time and location of most major earthquakes can be predicted several  |
|           | days in advance   |
| Option C: | Earthquakes are caused by heavy winds                                     |
| Option D: | P waves travel faster   |
|           |   |
| 5.3       | New Zealand is an example of  |
| Option A: | Convergent plate boundary   |
| Option B: | Divergent plate boundary  |
| Option C: | Conservative plate boundary   |
| Option D: | Both convergent and conservative plate boundaries                         |
|           |   |
| 5.4       | Love waves cause motion similar to S waves                                |
| Option A: | With vertical component   |
| Option B: | Without vertical component  |
| Option C: | With inclined component   |
| Option D: | Without inclined component at 45 degrees                                  |
|           |   |
| 5.5       | Mercalli indices of VII or higher measure the effects of an earthquake on |
| Option A: | cows  |
| Option B: | horses  |
| Option C: | people  |
| Option D: | Buildings   |
|           |   |
| 5.6       | Surface along which the block of rock slip is called?                     |
| Option A: | Fault zone  |
| Option B: | Fault Plane   |
| Option C: | Fault scarp   |
| Option D: | None of these   |
|           |   |
|           |   |

| 5.7       | On a seismic record, the S-P time interval is the in arrival time              |
|-----------|--|
|           | between the P- and S waves.  |
| Option A: | DELAY  |
| Option B: | Twice the delay  |
| Option C: | Four times the delay   |
| Option D: | Five times the delay   |
|           |  |
| 5.8       | Given three differently located seismic stations, the time-travel graph can be |
|           | used to determine the position of the  |
| Option A: | Epicentre  |
| Option B: | Radius of earth  |
| Option C: | Elasticity   |
| Option D: | Mass of earth  |
|           |  |
| 5.9       | From the S-P interval a seismologists can determine the to an                  |
|           | earthquake.  |
|           |  |
| Option A: | Distance   |
| Option B: | Earthquake force   |
| Option C: | Mass of earth  |
| Option D: | Elasticity   |
|           |  |
| 5.10      | While considering the design of R.C. buildings for providing ductility, IS     |
|           | codes prohibit the steel grade greater than                                    |
| Option A: | Fe 250   |
| Option B: | Fe 320   |
| Option C: | Fe 415   |
| Option D: | Fe 550   |
|           |  |
| 5.11      | The height of building is 10.5m. base dimension is 8m. the fundamental         |
|           | natural period of vibration is   |
| Option A: | 0.334 sec  |
| Option B: | 0.9 sec  |

| Option D: 2.1 sec  5.12 Now India is divided into   | Option C: | 1.5 sec   |
|---|-----------|---|
| Option A: 1 Option B: 2 Option C: 3 Option D: 4  5.13 During an eathquke which of following may be generated Option A: Draught Option B: Tsunami Option C: Heavy rains Option D: Low temperatures  5.14 Which of the following is depends on shear strength of the material? Option A: Density of material Option B: Internal friction Option C: Position of material Option D: Mass of the material  5.15 As rupture along a fault initiates, waves of energy travel outward from the hypocenter in a:  Option A: linear fashion, Option B: linear fashion, Option B: linear fashion, Option C: a spherical fashion, Option D: none of the above  5.16 At a seismic station the first waves to arrive are Option A: P Wave | Option D: | 2.1 sec   |
| Option A: 1 Option B: 2 Option C: 3 Option D: 4  5.13 During an eathquke which of following may be generated Option A: Draught Option B: Tsunami Option C: Heavy rains Option D: Low temperatures  5.14 Which of the following is depends on shear strength of the material? Option A: Density of material Option B: Internal friction Option C: Position of material Option D: Mass of the material  5.15 As rupture along a fault initiates, waves of energy travel outward from the hypocenter in a:  Option A: linear fashion, Option B: linear fashion, Option B: linear fashion, Option C: a spherical fashion, Option D: none of the above  5.16 At a seismic station the first waves to arrive are Option A: P Wave |           |   |
| Option B: 2 Option C: 3 Option D: 4  5.13 During an eathquke which of following may be generated Option A: Draught Option B: Tsunami Option C: Heavy rains Option D: Low temperatures  5.14 Which of the following is depends on shear strength of the material? Option A: Density of material Option B: Internal friction Option C: Position of material Option D: Mass of the material  5.15 As rupture along a fault initiates, waves of energy travel outward from the hypocenter in a:  Option A: linear fashion, Option B: linear fashion, Option C: a spherical fashion, Option C: none of the above  5.16 At a seismic station the first waves to arrive are Option A: P Wave                                       | 5.12      | Now India is divided into seismic zones.                                    |
| Option C: 3 Option D: 4  5.13 During an eathquke which of following may be generated Option A: Draught Option B: Tsunami Option C: Heavy rains Option D: Low temperatures  5.14 Which of the following is depends on shear strength of the material? Option A: Density of material Option B: Internal friction Option C: Position of material Option D: Mass of the material  5.15 As rupture along a fault initiates, waves of energy travel outward from the hypocenter in a:  Option A: linear fashion, Option B: linear fashion, Option C: a spherical fashion, Option C: a spherical fashion, Option C: none of the above  5.16 At a seismic station the first waves to arrive are Option A: P Wave                    | Option A: | 1   |
| Option D: 4  5.13 During an eathquke which of following may be generated Option A: Draught Option B: Tsunami Option C: Heavy rains Option D: Low temperatures  5.14 Which of the following is depends on shear strength of the material? Option A: Density of material Option B: Internal friction Option C: Position of material Option D: Mass of the material  5.15 As rupture along a fault initiates, waves of energy travel outward from the hypocenter in a:  Option A: linear fashion, Option B: linear fashion Option C: a spherical fashion, Option D: none of the above  5.16 At a seismic station the first waves to arrive are Option A: P Wave  | Option B: | 2   |
| 5.13 During an eathquke which of following may be generated Option A: Draught Option B: Tsunami Option C: Heavy rains Option D: Low temperatures  5.14 Which of the following is depends on shear strength of the material? Option A: Density of material Option B: Internal friction Option C: Position of material Option D: Mass of the material  5.15 As rupture along a fault initiates, waves of energy travel outward from the hypocenter in a:  Option A: linear fashion, Option B: linear fashion Option C: a spherical fashion, Option D: none of the above  5.16 At a seismic station the first waves to arrive are Option A: P Wave   | Option C: | 3   |
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| Option C: Position of material  Option D: Mass of the material  5.15 As rupture along a fault initiates, waves of energy travel outward from the hypocenter in a:  Option A: linear fashion,  Option B: linear fashion  Option C: a spherical fashion,  Option D: none of the above  5.16 At a seismic station the first waves to arrive are  Option A: P Wave  | Option A: | Density of material   |
| Option D: Mass of the material  5.15 As rupture along a fault initiates, waves of energy travel outward from the hypocenter in a:  Option A: linear fashion, Option B: linear fashion Option C: a spherical fashion, Option D: none of the above  5.16 At a seismic station the first waves to arrive are Option A: P Wave  | Option B: | Internal friction   |
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| Option A: linear fashion, Option B: linear fashion Option C: a spherical fashion, Option D: none of the above  5.16 At a seismic station the first waves to arrive are Option A: P Wave   | 5.15      | As rupture along a fault initiates, waves of energy travel outward from the |
| Option B: linear fashion  Option C: a spherical fashion,  Option D: none of the above  5.16 At a seismic station the first waves to arrive are  Option A: P Wave  |           | hypocenter in a:  |
| Option B: linear fashion  Option C: a spherical fashion,  Option D: none of the above  5.16 At a seismic station the first waves to arrive are  Option A: P Wave  |           |   |
| Option C: a spherical fashion, Option D: none of the above  5.16 At a seismic station the first waves to arrive are Option A: P Wave  | Option A: | linear fashion,   |
| Option D: none of the above  5.16 At a seismic station the first waves to arrive are Option A: P Wave   | Option B: | linear fashion  |
| 5.16 At a seismic station the first waves to arrive are  Option A: P Wave   | Option C: | a spherical fashion,  |
| Option A: P Wave  | Option D: | none of the above   |
| Option A: P Wave  |           |   |
| 1   | 5.16      | At a seismic station the first waves to arrive are                          |
| Option B: S Wave  | Option A: | P Wave  |
|   | Option B: | S Wave  |

| Option D: Love wave  5.17 At a seismic station the last waves to arrive are Option A: P Wave Option B: S Wave Option C: Surface wave Option D: light ray waves  5.18 Love waves are . Option A: Dispersive Option B: Displace material in elliptical path Option C: Fastest among all Option D: Are principal component of ground roll  5.19 are the most destructive to buildings.  Option A: P Wave Option B: S Wave Option B: S Wave Option C: P waves are two times than S wave Option D: P and S wave similar  5.20  | Option C: | Surface wave   |
|---|-----------|--|
| Option A: P Wave Option B: S Wave Option C: Surface wave Option D: light ray waves  5.18  | Option D: | Love wave  |
| Option A: P Wave Option B: S Wave Option C: Surface wave Option D: light ray waves  5.18  |           |  |
| Option A: P Wave Option B: S Wave Option C: Surface wave Option D: light ray waves  5.18  |           |  |
| Option B: S Wave Option C: Surface wave Option D: light ray waves  5.18    Love waves are . Option A: Dispersive Option B: Displace material in elliptical path Option C: Fastest among all Option D: Are principal component of ground roll  5.19    are the most destructive to buildings. Option A: P Wave Option B: S Wave Option B: S Wave Option C: P waves are two times than S wave Option D: P and S wave similar  5.20    Each unit increase in magnitude on the Richter scale corresponds to an increase in seismic activity.  Option A: 10 Option B: 100 Option C: 50 Option C: 50 Option C: 50 Option A: 30,000 times annually | 5.17      | At a seismic station the last waves to arrive are                      |
| Option C: Surface wave Option D: light ray waves  5.18  | Option A: | P Wave   |
| Option D: light ray waves  5.18   | Option B: | S Wave   |
| 5.18 Love waves are .  Option A: Dispersive  Option B: Displace material in elliptical path  Option C: Fastest among all  Option D: Are principal component of ground roll  5.19 are the most destructive to buildings.  Option A: P Wave  Option B: S Wave  Option C: P waves are two times than S wave  Option D: P and S wave similar  5.20 Each unit increase in magnitude on the Richter scale corresponds to an increase in seismic activity.  Option A: 10  Option B: 100  Option C: 50  Option D: 25  5.21 Great earthquakes, on average, occur  Option A: 30,000 times annually  | Option C: | Surface wave   |
| Option A: Dispersive Option B: Displace material in elliptical path Option C: Fastest among all Option D: Are principal component of ground roll  5.19 are the most destructive to buildings. Option A: P Wave Option B: S Wave Option C: P waves are two times than S wave Option D: P and S wave similar  5.20 Each unit increase in magnitude on the Richter scale corresponds to an increase in seismic activity. Option A: 10 Option B: 100 Option C: 50 Option C: 50 Option D: 25  5.21 Great earthquakes, on average, occur Option A: 30,000 times annually  | Option D: | light ray waves  |
| Option A: Dispersive Option B: Displace material in elliptical path Option C: Fastest among all Option D: Are principal component of ground roll  5.19 are the most destructive to buildings. Option A: P Wave Option B: S Wave Option C: P waves are two times than S wave Option D: P and S wave similar  5.20 Each unit increase in magnitude on the Richter scale corresponds to an increase in seismic activity. Option A: 10 Option B: 100 Option C: 50 Option C: 50 Option D: 25  5.21 Great earthquakes, on average, occur Option A: 30,000 times annually  |           |  |
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| Option D: Are principal component of ground roll  5.19 are the most destructive to buildings.  Option A: P Wave  Option B: S Wave  Option C: P waves are two times than S wave  Option D: P and S wave similar  5.20 Each unit increase in magnitude on the Richter scale corresponds to an increase in seismic activity.  Option A: 10  Option B: 100  Option C: 50  Option C: 50  Option D: 25  5.21 Great earthquakes, on average, occur  Option A: 30,000 times annually  | Option B: | Displace material in elliptical path                                   |
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| Option A: P Wave Option B: S Wave Option C: P waves are two times than S wave Option D: P and S wave similar  5.20 Each unit increase in magnitude on the Richter scale corresponds to an increase in seismic activity.  Option A: 10 Option B: 100 Option C: 50 Option D: 25  5.21 Great earthquakes, on average, occur Option A: 30,000 times annually  |           |  |
| Option B: S Wave  Option C: P waves are two times than S wave  Option D: P and S wave similar  5.20 Each unit increase in magnitude on the Richter scale corresponds to an increase in seismic activity.  Option A: 10  Option B: 100  Option C: 50  Option D: 25  5.21 Great earthquakes, on average, occur  Option A: 30,000 times annually   | 5.19      | are the most destructive to buildings.                                 |
| Option C: P waves are two times than S wave  Option D: P and S wave similar  5.20 Each unit increase in magnitude on the Richter scale corresponds to an increase in seismic activity.  Option A: 10  Option B: 100  Option C: 50  Option D: 25  5.21 Great earthquakes, on average, occur  Option A: 30,000 times annually   | Option A: | P Wave   |
| Option D: P and S wave similar  5.20 Each unit increase in magnitude on the Richter scale corresponds to an increase in seismic activity.  Option A: 10  Option B: 100  Option C: 50  Option D: 25  5.21 Great earthquakes, on average, occur  Option A: 30,000 times annually  | Option B: | S Wave   |
| 5.20 Each unit increase in magnitude on the Richter scale corresponds to an increase in seismic activity.  Option A: 10  Option B: 100  Option C: 50  Option D: 25  5.21 Great earthquakes, on average, occur  Option A: 30,000 times annually  | Option C: | P waves are two times than S wave                                      |
| increase in seismic activity.  Option A: 10  Option B: 100  Option C: 50  Option D: 25  5.21 Great earthquakes, on average, occur  Option A: 30,000 times annually  | Option D: | P and S wave similar   |
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| Option A: 10 Option B: 100 Option C: 50 Option D: 25  5.21 Great earthquakes, on average, occur Option A: 30,000 times annually   | 5.20      | Each unit increase in magnitude on the Richter scale corresponds to an |
| Option B: 100 Option C: 50 Option D: 25  5.21 Great earthquakes, on average, occur Option A: 30,000 times annually  |           | increase in seismic activity.  |
| Option C: 50 Option D: 25  5.21 Great earthquakes, on average, occur Option A: 30,000 times annually  | Option A: | 10   |
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| 5.21 Great earthquakes, on average, occur  Option A: 30,000 times annually  | Option C: | 50   |
| Option A: 30,000 times annually   | Option D: | 25   |
| Option A: 30,000 times annually   |           |  |
|   | 5.21      | Great earthquakes, on average, occur                                   |
| Option B: 500 times annually  | Option A: | 30,000 times annually  |
| i l   | Option B: | 500 times annually   |

| Option C: | 20 times annually   |
|-----------|---|
| Option D: | once every 5 to 10 years  |
|           |   |
| 5.22      | The modified Mercalli scale varies from to  |
| Option A: | I to XII  |
| Option B: | I to X  |
| Option C: | I to VII  |
| Option D: | I to IV   |
|           |   |
|           | MODULE 6 PRESTRESSED CONCRETE   |
|           |   |
| 6.1       | A post tensioned beam has span of 25m. If the slip at the jacking end is 4            |
|           | mm, and E=210 kN/mm <sup>2</sup> , the percentage loss of stress due to this cause is |
| Option A: | 12.2 N/mm <sup>2</sup>  |
|           |   |
| Option B: | 33.6 N/mm <sup>2</sup>  |
| Option C: | 18.3 N/mm <sup>2</sup>  |
|           | 2.  |
| Option D: | 54.7 N/mm <sup>2</sup>  |
|           |   |
| 6.2       | When the prestressing cable is passing through upper kern point                       |
| Option A: | the stress at the lower fibre of the beam is zero.                                    |
| Option B: | the stress at the lower kern point is zero.   |
| Option C: | the stress at the centroidal axis is zero.  |
| Option C: | the stress at the top fibre of the beam is zero.                                      |
| Орион D.  | the stress at the top flore of the beam is zero.                                      |
| 6.3       | The concept of load balancing is useful in selecting?                                 |
| 0.5       | The concept of load balancing is useful in selecting:                                 |
| Option A: | Anchorage profile   |
| Option B: | Shaft profile   |
|           | r   |
| Option C: | Tendon profile  |
|           |   |

| Option D: | Span profile  |
|-----------|---|
| 6.4       | A prestressed concrete beam is loaded with two point loads .The profile of the cable is laid based on the load balancing concept, the shape of profile is       |
| Option A: | Parabolic   |
| Option B: | Triangular  |
| Option C: | Trapezoid   |
| Option D: | Circular  |
| 6.5       | From the following which steel grade is recommended as tendons for post tensioned concrete girder.  |
| Option A: | Fe 250  |
| Option B: | Fe 415  |
| Option C: | Fe 275  |
| Option D: | Fe 1500   |
| 6.6       | The pressure line is also known as  |
| Option A: | C line  |
| Option B: | E line  |
| Option C: | G line  |
| Option D: | I line  |
| 6.7       | If in a post tensioned beam the age of concrete at prestress transfer is 7 days. If $E=210~kN/mm^2$ , the loss in prestress due to residual shrinkage strain is |
| Option A: | 44 N/mm <sup>2</sup>  |
| Option B: | $8 \text{ N/mm}^2$  |

| 2 N/mm <sup>2</sup>  |
|--|
|  |
| The change in the external moments in the elastic range of prestressed   |
| oncrete beam results in  |
| Bending moment in pressure line  |
| Corsion in pressure line   |
| Elexure in pressure line   |
| shift of the pressure line   |
|  |
| The method of prestressing the concrete after it attains its strength is |
| nown as  |
| Pre tensioning   |
| Post tensioning  |
| Chemical prestressing  |
| Axial prestressing   |
|  |
| From the following which concrete grade is recommended for posttensioned |
| oncrete girder.  |
| M 20   |
|  |
| M 40   |
| M 15   |
| A 25   |
|  |
| The frictional and anchorage slip losses are observed in                 |
| Post tensioned members   |
| Pre tensioned members  |
|  |

| Option C: | Ruptured members  |
|-----------|---|
| Option D: | Axial member  |
|           |   |
| 6.12      | In which method the prestress is developed due to the bond between the          |
|           | concrete and steel?   |
| Option A: | Pre tensioning  |
| Option B: | Post tensioning   |
| Option C: | Thermo electric prestressing  |
| Option D: | Prefix beam prestressing  |
|           |   |
| 6.13      | A rectangular prestressed concrete beam 400mm*600mm is subjected to BM          |
|           | of 72kNm. If the axial prestreesing force is 960 kN, the extreme fibre stresses |
|           | in N/mm <sup>2</sup> are  |
| Option A: | 7 N/mm <sup>2</sup> and 1 N/mm <sup>2</sup>                                     |
| Option B: | 4 N/mm <sup>2</sup> and 5 N/mm <sup>2</sup>                                     |
| Option C: | 4 N/mm <sup>2</sup> and 9 N/mm <sup>2</sup>                                     |
| Option D: | 5 N/mm <sup>2</sup> and 1 N/mm <sup>2</sup>                                     |
|           |   |
| 6.14      | The tendons in the pretensioning system are tensioned between                   |
| Option A: | Rigid anchorages  |
| Option B: | Hydraulic jacks   |
| Option C: | Concrete beds   |
| Option D: | Variable beams  |
|           |   |
| 6.15      | Which is one of the systems used for pretensioning                              |
| Option A: | Magnel-Balton system  |
| Option B: | Freyssinet system   |
| Option C: | Gifford-Udall system  |
| Option D: | Hoyer's long line method  |
|           |   |
| L         | •   |